

## 4. Symmetrical and Unsymmetrical Bending

### 4.1 Internal Forces in Beams

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### 4.3 Unsymmetric Bending of Straight Beams

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- Case of Transverse Bending

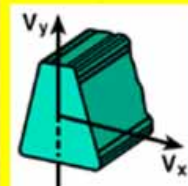
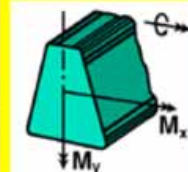
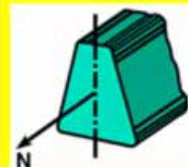
## Internal Forces in Beams

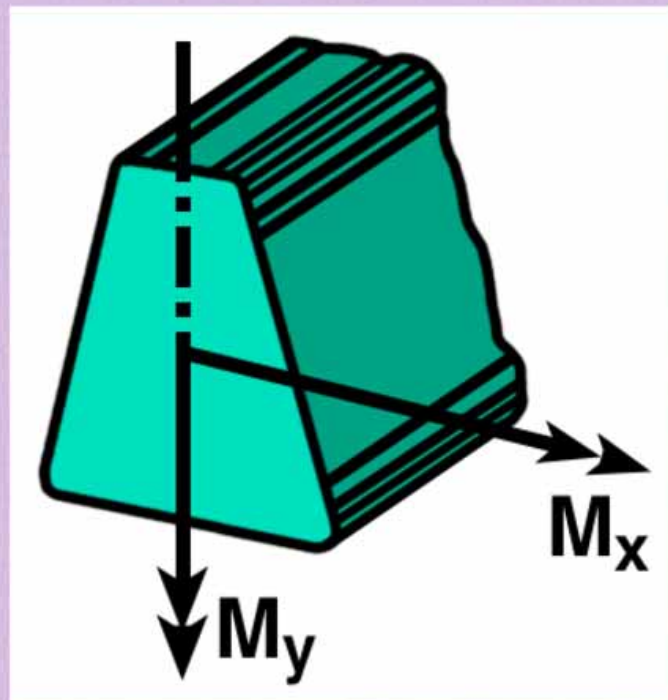
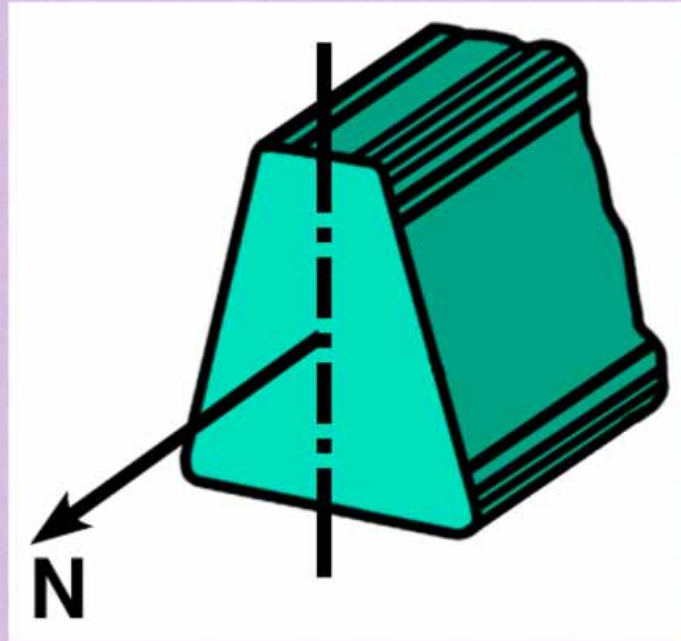
### Forces Associated with Normal Stresses

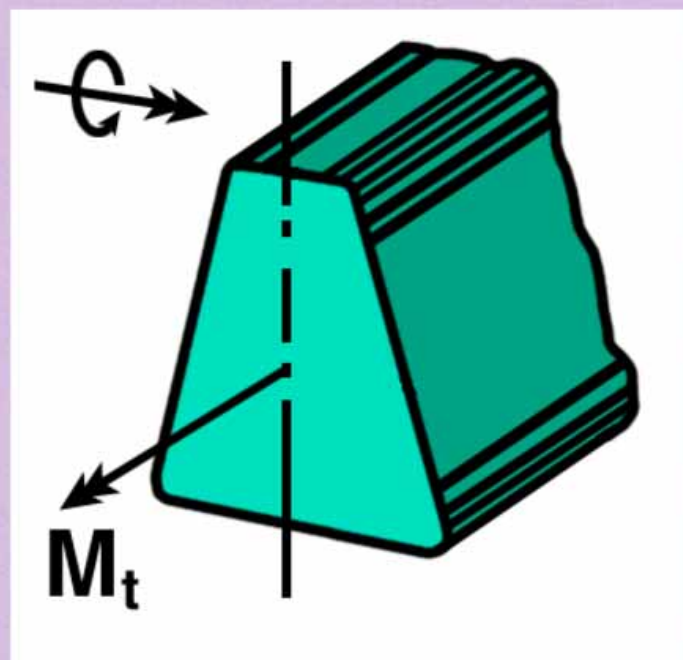
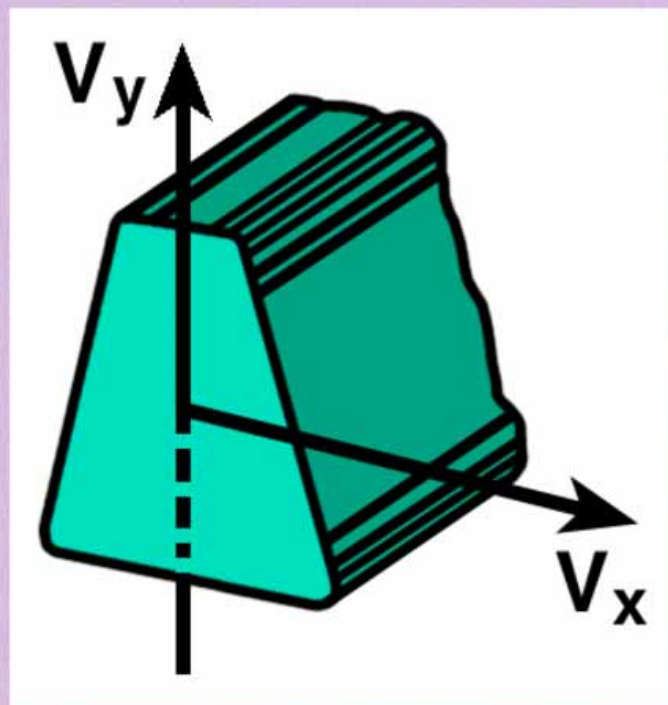
Normal (or axial) force	$N$
Bending moment (plane $yz$ )	$M_x$
Bending moment (plane $xz$ )	$M_y$

### Forces Associated with Shear Stresses

Shearing force	$V_x$
Shearing force	$V_y$
Twisting moment (plane $xy$ )	$M_t$



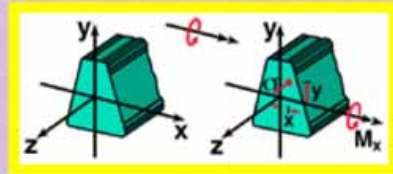






## Simple Formula for Normal Stresses Due to Pure (or Transverse) Bending

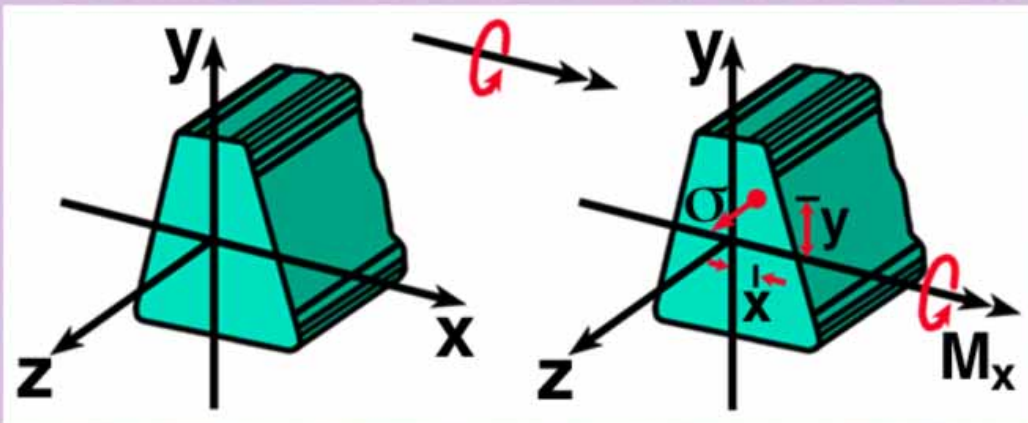
$$\sigma = \frac{M_x y}{I_x}$$



### Assumptions

- Plane of loads (and of bending) is perpendicular to neutral axis
- x axis is the neutral axis and is a principal centroidal axis

$$S_x = \int_A y \, dA = 0 \quad , \quad I_{xy} = \int_A xy \, dA = 0$$



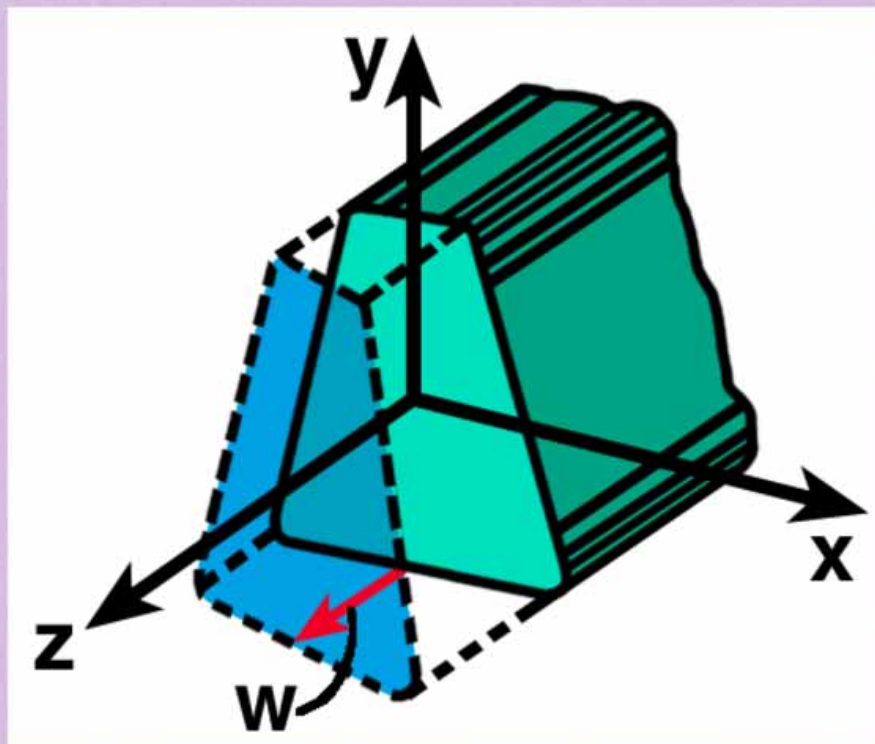
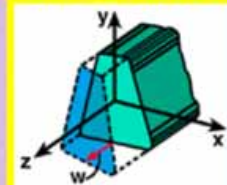
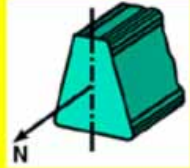
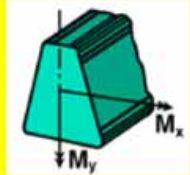
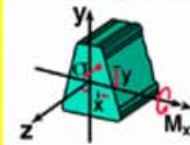
## Simple Formula for Normal Stresses Due to Pure (or Transverse) Bending

### Internal Forces at the Cross Section

- Normal force  $N$
- Bending moments  $M_x$  and  $M_y$  about the  $x$  and  $y$  axes

### Kinematic Relations

- Plane cross section before deformation is assumed to remain plane after deformation



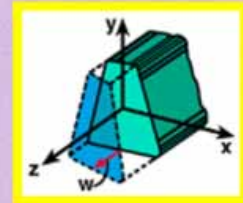


## Simple Formula for Normal Stresses Due to Pure (or Transverse) Bending

### Kinematic Relations

- Plane cross section before deformation is assumed to remain plane after deformation
- Both the displacement  $w$  in the axial direction and the strain  $\varepsilon$  are linear functions of  $x$  and  $y$

$$\varepsilon = \frac{\partial w}{\partial z} = \begin{bmatrix} 1 & x & y \end{bmatrix} \begin{Bmatrix} a \\ b \\ c \end{Bmatrix}$$



$a, b, c$  are independent of  $x$  and  $y$

## Simple Formula for Normal Stresses Due to Pure (or Transverse) Bending

### Static Relations

- Sum of internal stresses in the axial direction

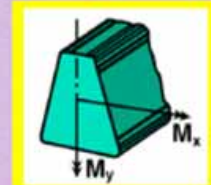
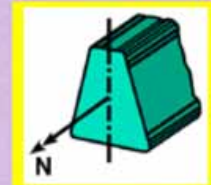
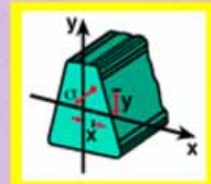
$$\int_A \sigma \, dA = N$$

- Sum of moments of internal stresses about  $y$  axis

$$\int_A \sigma x \, dA = M_y$$

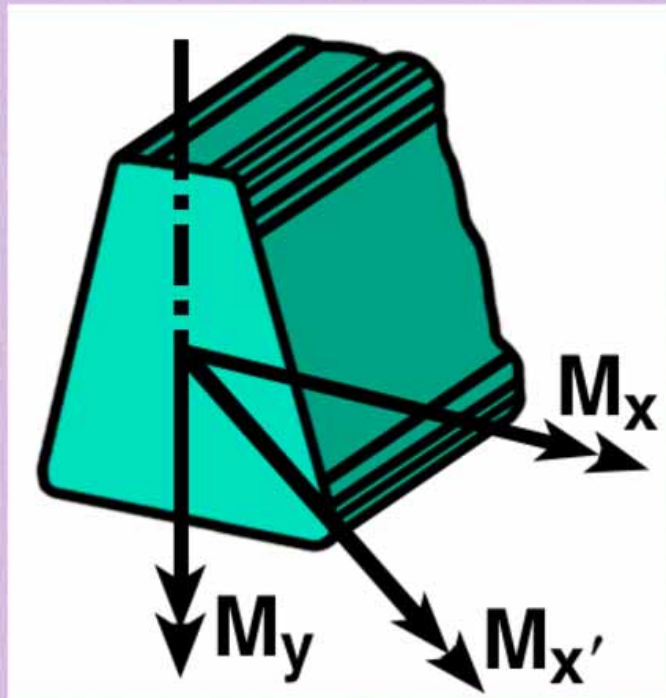
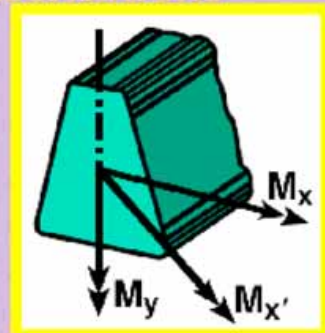
- Sum of moments of internal stresses about  $x$  axis

$$\int_A \sigma y \, dA = M_x$$



## Unsymymmetric Bending of Straight Beams

- When the plane of bending is not a principal plane, then
  - Either use a more general formula for the normal stresses (resulting from a bending moment  $M_{x'}$ ), or
  - Decompose the bending moment into components whose vector representations are along principal centroidal axes





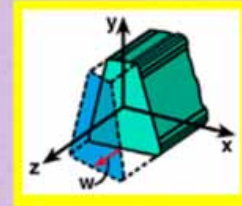
## Case of Combined Normal Force and Bending Moments

### Constitutive Relations

- For linearly elastic material - uniaxial stress state

$$\sigma = E \varepsilon$$

$$= E \begin{bmatrix} 1 & x & y \end{bmatrix} \begin{Bmatrix} a \\ b \\ c \end{Bmatrix}$$



- From the static relations

$$E \begin{bmatrix} A & S_y & S_x \\ S_y & I_y & I_{xy} \\ S_x & I_{xy} & I_x \end{bmatrix} \begin{Bmatrix} a \\ b \\ c \end{Bmatrix} = \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$

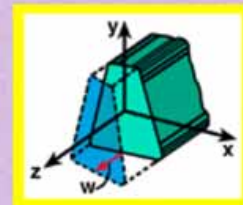
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## Case of Combined Normal Force and Bending Moments

- From the static relations

$$E \begin{bmatrix} A & S_y & S_x \\ S_y & I_y & I_{xy} \\ S_x & I_{xy} & I_x \end{bmatrix} \begin{Bmatrix} a \\ b \\ c \end{Bmatrix} = \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$

+



where

$$\begin{Bmatrix} A \\ S_x \\ S_y \end{Bmatrix} = \int_A \begin{Bmatrix} 1 \\ y \\ x \end{Bmatrix} dA ; \quad \begin{Bmatrix} I_x \\ I_y \\ I_{xy} \end{Bmatrix} = \int_A \begin{Bmatrix} y^2 \\ x^2 \\ xy \end{Bmatrix} dA$$

+



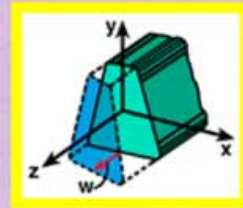
## Case of Combined Normal Force and Bending Moments

where

$$\begin{Bmatrix} A \\ S_x \\ S_y \end{Bmatrix} = \int_A \begin{Bmatrix} 1 \\ y \\ x \end{Bmatrix} dA ; \quad \begin{Bmatrix} I_x \\ I_y \\ I_{xy} \end{Bmatrix} = \int_A \begin{Bmatrix} y^2 \\ x^2 \\ xy \end{Bmatrix} dA$$

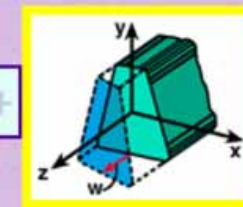
or

$$\begin{Bmatrix} a \\ b \\ c \end{Bmatrix} = \frac{1}{E} \begin{bmatrix} A & S_y & S_x \\ S_y & I_y & I_{xy} \\ S_x & I_{xy} & I_x \end{bmatrix}^{-1} \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$



## Case of Combined Normal Force and Bending Moments

$$\begin{Bmatrix} a \\ b \\ c \end{Bmatrix} = \frac{1}{E} \begin{bmatrix} A & S_y & S_x \\ S_y & I_y & I_{xy} \\ S_x & I_{xy} & I_x \end{bmatrix}^{-1} \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$



- The general formula for normal stresses in terms of the normal force and bending moments:

$$\sigma = [1 \quad x \quad y] \begin{bmatrix} A & S_y & S_x \\ S_y & I_y & I_{xy} \\ S_x & I_{xy} & I_x \end{bmatrix}^{-1} \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$

## Case of Combined Normal Force and Bending Moments

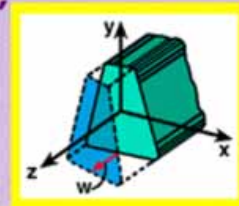
### Simplifications

- If  $x$  and  $y$  are centroidal axes, then

$$S_x = S_y = 0$$

$$\sigma = \begin{bmatrix} 1 & x & y \end{bmatrix} \begin{bmatrix} A & 0 & 0 \\ 0 & I_y & I_{xy} \\ 0 & I_{xy} & I_x \end{bmatrix}^{-1} \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$

- If  $x$  and  $y$  are centroidal principal axes, then  $S_x = S_y = 0$ ,  $I_{xy} = 0$

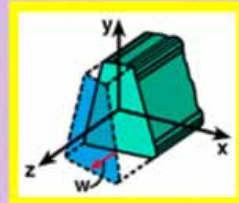


## Case of Combined Normal Force and Bending Moments

- If  $x$  and  $y$  are centroidal principal axes, then  $S_x = S_y = 0$ ,  $I_{xy} = 0$

$$\sigma = \begin{bmatrix} 1 & x & y \end{bmatrix} \begin{bmatrix} \frac{1}{A} & \cdot & \cdot \\ \cdot & \frac{1}{I_y} & \cdot \\ \cdot & \cdot & \frac{1}{I_x} \end{bmatrix} \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$

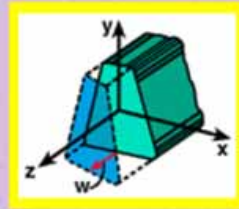
$$\sigma = \frac{N}{A} + \frac{M_y}{I_y} x + \frac{M_x}{I_x} y$$





## Case of Combined Normal Force and Bending Moments

$$\sigma = \frac{N}{A} + \frac{M_y}{I_y} x + \frac{M_x}{I_x} y$$



**Neutral Axis**

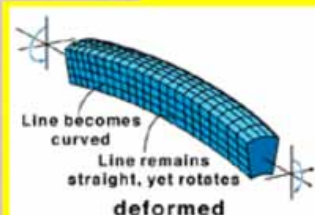
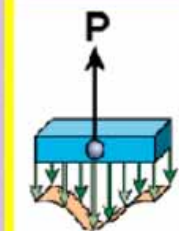
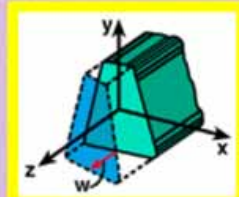
Is the axis at which the normal stress  $\sigma = 0$

## Case of Combined Normal Force and Bending Moments

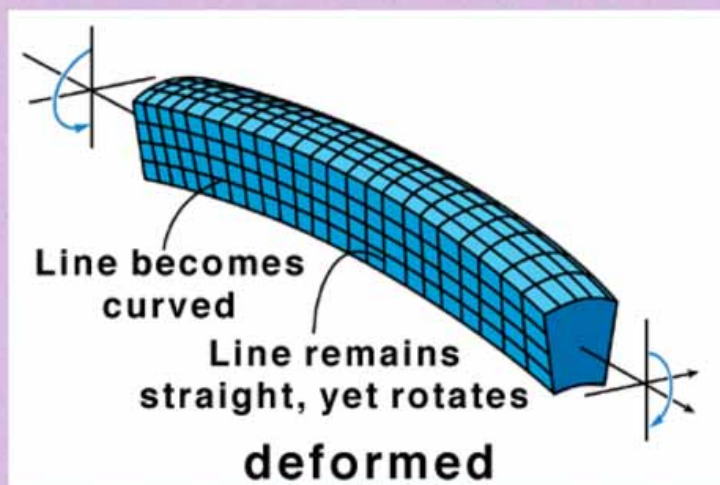
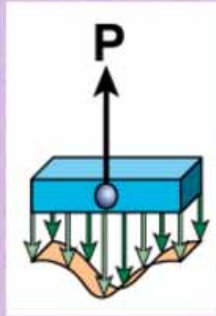
- Assumption of plane cross section before deformation remains plane
  - Is accurate for:
    - axial load (away from point of application of load)
    - pure bending
  - Is approximate for transverse bending
- Assumption of undeformable cross sections

$$\epsilon_x = \epsilon_y = \gamma_{xy} = 0$$

- Only approximate for bending



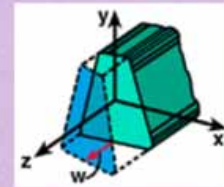




## Case of Combined Normal Force and Bending Moments

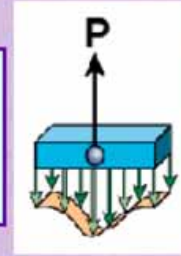
- Assumption of undeformable cross sections

$$\epsilon_x = \epsilon_y = \gamma_{xy} = 0$$



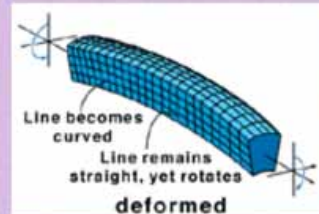
- Only approximate for bending

$$(\epsilon_x, \epsilon_y, \gamma_{xy}) \ll \epsilon_z$$



- More approximate than for beams under axial loading

$$\epsilon_x = \epsilon_y = -\nu \epsilon_z$$



## Calculations of Displacements

Rotation, Curvature and Axial Strain

- For the case of a single bending moment  $M_x$

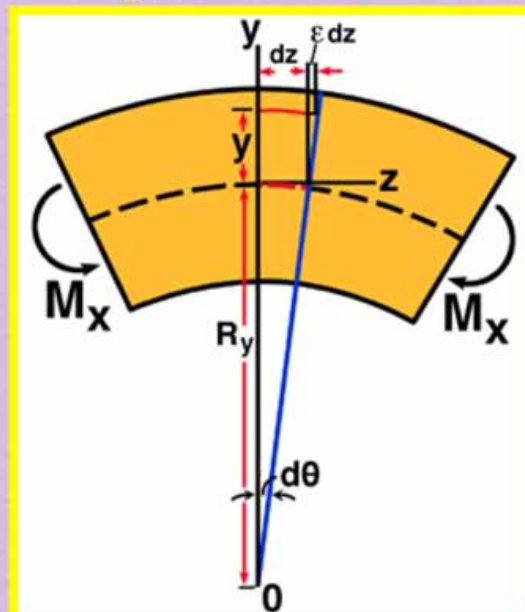
$$d\theta = \frac{dz}{R_y} = \frac{\epsilon}{y} dz$$

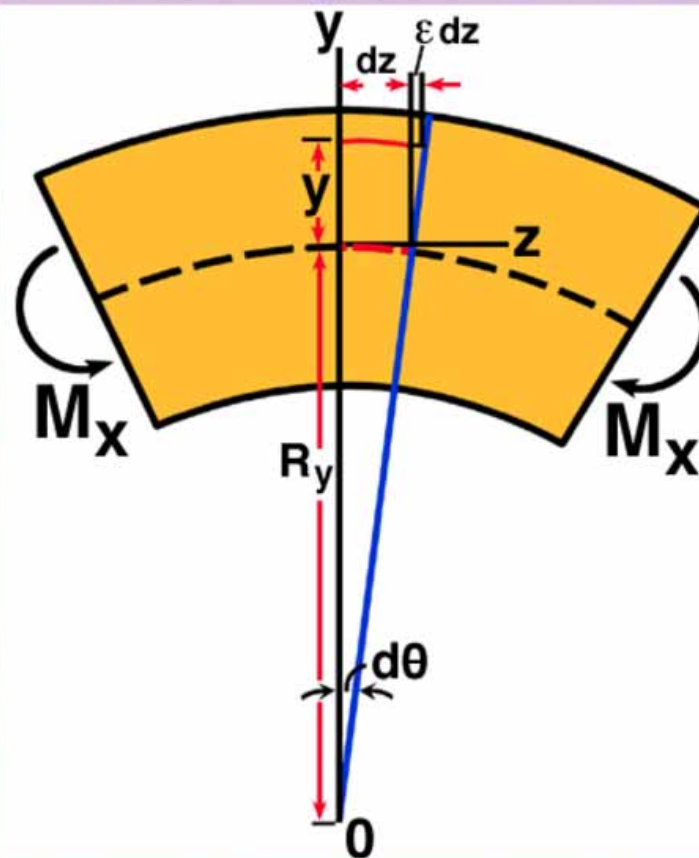
$$\frac{1}{R_y} = \frac{d\theta}{dz} = \frac{\epsilon}{y}$$

$$\approx -\frac{d^2v}{dz^2}$$

or

$$\epsilon \approx -y \frac{d^2v}{dz^2}$$





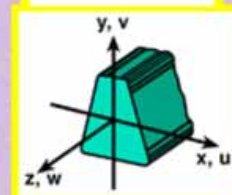
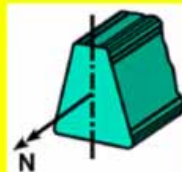
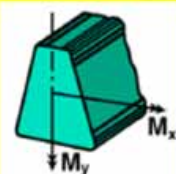
## Calculations of Displacements

- Analogously, for a bending moment  $M_y$

$$\varepsilon \simeq -x \frac{d^2 u}{dz^2}$$

- And, for an axial force  $N$
- For the case of combined axial force, bending moment  $M_x$  and bending moment  $M_y$

$$\varepsilon = [1 \ x \ y] \begin{Bmatrix} \varepsilon_0 \\ -\frac{d^2 u}{dz^2} \\ -\frac{d^2 v}{dz^2} \end{Bmatrix}$$



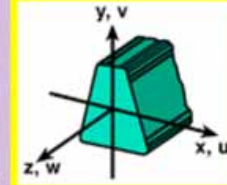
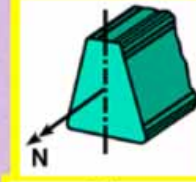
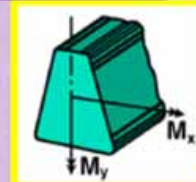


## Calculations of Displacements

Displacement equations  
from which

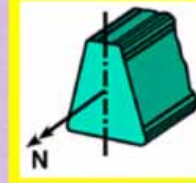
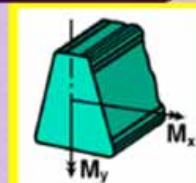
$$\varepsilon = \frac{dw}{dz} = \frac{\sigma}{E}$$

$$\begin{Bmatrix} \varepsilon_o \\ -\frac{d^2u}{dz^2} \\ -\frac{d^2v}{dz^2} \end{Bmatrix} = \frac{1}{E} \begin{bmatrix} A & S_y & S_x \\ S_y & I_y & I_{xy} \\ S_x & I_{xy} & I_x \end{bmatrix}^{-1} \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$



## Calculations of Displacements

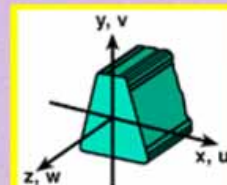
$$\begin{Bmatrix} \varepsilon_o \\ -\frac{d^2u}{dz^2} \\ -\frac{d^2v}{dz^2} \end{Bmatrix} = \frac{1}{E} \begin{bmatrix} A & S_y & S_x \\ S_y & I_y & I_{xy} \\ S_x & I_{xy} & I_x \end{bmatrix}^{-1} \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$



Simplifications

- If x and y are centroidal axes, and N is absent, then

$$\begin{Bmatrix} \frac{d^2u}{dz^2} \\ \frac{d^2v}{dz^2} \end{Bmatrix} = \frac{-1}{E} \begin{bmatrix} I_y & I_{xy} \\ I_{xy} & I_x \end{bmatrix}^{-1} \begin{Bmatrix} M_y \\ M_x \end{Bmatrix}$$

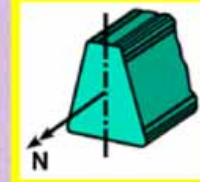
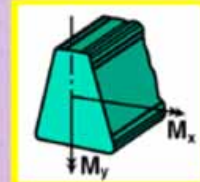


# Calculations of Displacements

## Simplifications

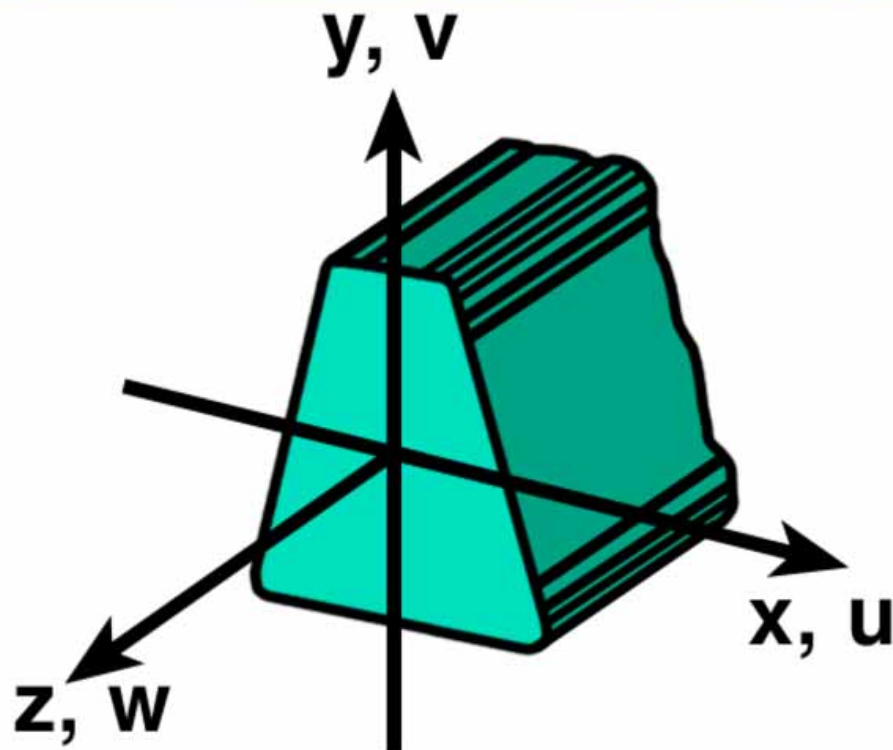
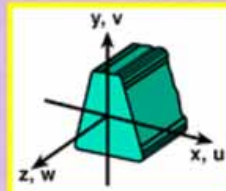
- If  $x$  and  $y$  are centroidal axes, and  $N$  is absent, then

$$\begin{Bmatrix} \frac{d^2u}{dz^2} \\ \frac{d^2v}{dz^2} \end{Bmatrix} = \frac{-1}{E} \begin{bmatrix} I_y & I_{xy} \\ I_{xy} & I_x \end{bmatrix}^{-1} \begin{Bmatrix} M_y \\ M_x \end{Bmatrix}$$



- If  $x$  and  $y$  are centroidal principal axes, and  $N$  is absent, then

$$\begin{Bmatrix} \frac{d^2u}{dz^2} \\ \frac{d^2v}{dz^2} \end{Bmatrix} = \frac{-1}{E} \begin{Bmatrix} \frac{M_y}{I_y} \\ \frac{M_x}{I_x} \end{Bmatrix}$$



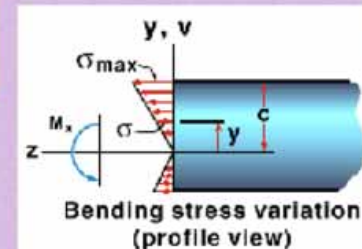
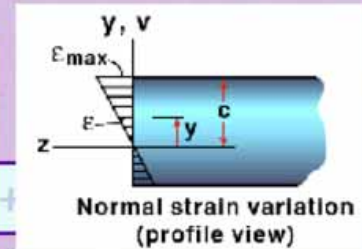
# Calculations of Displacements

## Case of Transverse Bending

- Governing equation for the elementary theory of beams:

$$\frac{d^2}{dz^2} \left( EI_x \frac{d^2 v}{dz^2} \right) = p_y$$

- For uniform beams



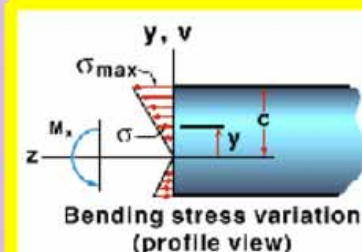
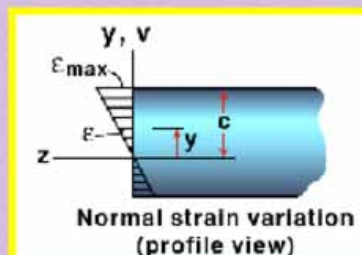
# Calculations of Displacements

## Case of Transverse Bending

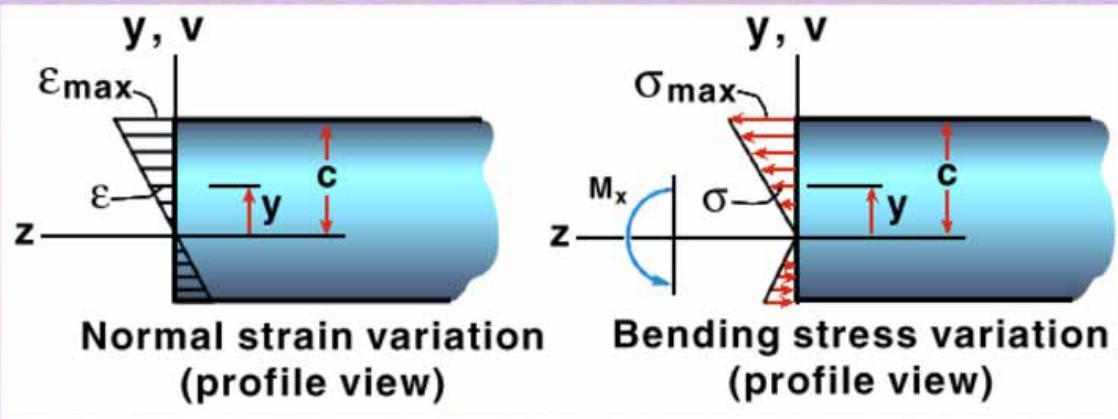
- For uniform beams

$$EI \frac{d^4 v}{dz^4} = p$$

where subscripts x and y have been dropped for convenience.







## Calculations of Displacements

### Case of Transverse Bending

- Successive integration of the differential equation

Transverse shear

$$EI \frac{d^3 v}{dz^3} = \int_0^z p \, dz + c_1 = -V \quad +$$

## Calculations of Displacements

Case of Transverse Bending

+

Bending Moment

$$EI \frac{d^2 v}{dz^2} = \int_0^z \int_0^z p \, dz \, dz + c_1 z + c_2$$
$$= -M$$

## Calculations of Displacements

Case of Transverse Bending

Slope

$$EI \frac{dv}{dz} = \int_0^z \int_0^z \int_0^z p \, dz \, dz \, dz$$
$$+ \frac{1}{2} c_1 z^2 + c_2 z + c_3$$

## Calculations of Displacements

Case of Transverse Bending

Transverse Displacement

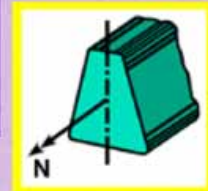
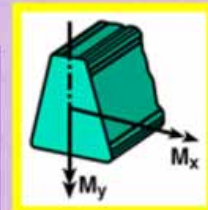
$$EI \ v = \int_0^z \int_0^z \int_0^z \int_0^z p \, dz \, dz \, dz \, dz$$

$$+ \frac{1}{6} c_1 z^3 + \frac{1}{2} c_2 z^2 + c_3 z + c_4$$

## Calculations of Displacements

- Case of combined  $N, M_x, M_y$

$$\sigma = [1 \ x \ y] \begin{bmatrix} A & S_y & S_x \\ S_y & I_x & I_{xy} \\ S_x & I_{xy} & I_y \end{bmatrix}^{-1} \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$

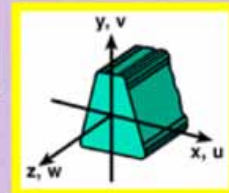
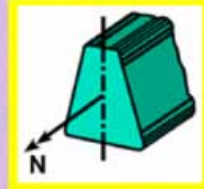
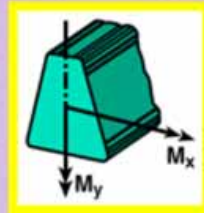




## Calculations of Displacements

- Displacement equations

$$\begin{Bmatrix} \varepsilon_o \\ -\frac{d^2u}{dz^2} \\ -\frac{d^2v}{dz^2} \end{Bmatrix} = \frac{1}{E} \begin{bmatrix} A & S_y & S_x \\ S_y & I_y & I_{xy} \\ S_x & I_{xy} & I_x \end{bmatrix}^{-1} \begin{Bmatrix} N \\ M_y \\ M_x \end{Bmatrix}$$



## Calculations of Displacements

- If  $x, y$  are centroidal principal axes

$$\frac{dw}{dz} = \frac{N}{EA}$$

$$-\frac{d^2u}{dz^2} = \frac{M_y}{EI_y} \quad -\frac{d^2v}{dz^2} = \frac{M_x}{EI_x}$$

$w$  is the displacement of the axis of the beam in the  $z$  direction

